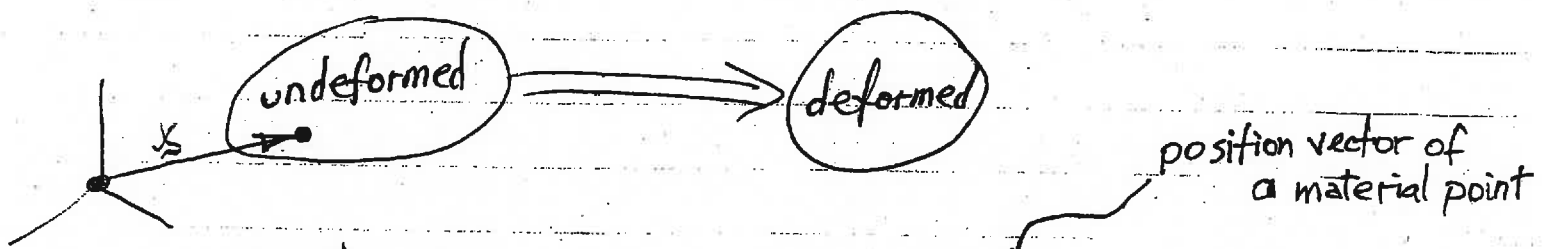


Strains



Displacement vector : $\underline{u} = \underline{u}(\underline{x})$

at neighbouring pt : $\underline{u}(\underline{x} + \Delta \underline{x})$

If $\Delta \underline{x}$ is small (local description, in a vicinity of \underline{x})

$$u_i(\underline{x} + \Delta \underline{x}) \doteq u_i(\underline{x}) + \frac{\partial u_i}{\partial x_j} \Delta x_j \quad (i=1,2,3)$$

or

$$\Delta u_i = \frac{\partial u_i}{\partial x_j} \Delta x_j$$

this matrix describes (local) non-uniformity of the displac. field

Components of Displacement gradient tensor

displacement
gradient tensor

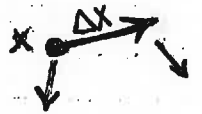
$$\underline{D} = \frac{\partial u_i}{\partial x_j} \underline{e}_i \underline{e}_j$$

so that

$$\Delta \underline{u} = \underline{D} \cdot \Delta \underline{x}$$

difference in displac.
of two points

Decompose into sym & antisym



$$\underline{D} = \underline{\epsilon} + \underline{\omega} = \left[\frac{1}{2} (D_{ij} + D_{ji}) + \frac{1}{2} (D_{ij} - D_{ji}) \right] \underline{e}_i \underline{e}_j$$

$$= \left[\frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right) + \frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} - \frac{\partial u_j}{\partial x_i} \right) \right] \underline{e}_i \underline{e}_j$$

$$\underbrace{\hspace{10em}}_{\epsilon_{ij}}$$

$$\underbrace{\hspace{10em}}_{\omega_{ij}}$$

$\underline{\epsilon}$ - strain tensor

$\underline{\omega}$ - rotation tensor

$$\overline{\Delta S^2} - \Delta S^2 = 2 \frac{\partial u_i}{\partial x_j} \Delta x_i \Delta x_j$$

$\underbrace{\hspace{10em}}$

$$\vartheta_{ij} = \epsilon_{ij} + \omega_{ij}$$

symm

antisymm

$$\Rightarrow \overline{\Delta S^2} - \Delta S^2 = 2 \epsilon_{ij} \Delta x_i \Delta x_j$$

$$+ 2 \omega_{ij} \Delta x_i \Delta x_j$$

$$\overline{\Delta S^2} - \Delta S^2 = 2 \frac{\partial u_i}{\partial x_j} \Delta x_i \Delta x_j$$

$$\underbrace{\frac{\partial u_i}{\partial x_j}}_{\mathcal{D}_{ij}} = \underbrace{\epsilon_{ij}}_{\text{symm}} + \underbrace{\omega_{ij}}_{\text{antisymm}}$$

$$\Rightarrow \overline{\Delta S^2} - \Delta S^2 = 2 \epsilon_{ij} \Delta x_i \Delta x_j$$

$$+ 2 \omega_{ij} \Delta x_i \Delta x_j$$

this term is = 0

$$\text{Indeed: } \omega_{11} = \omega_{22} = \omega_{33} = 0$$

$$\omega_{21} = -\omega_{12} \Rightarrow \omega_{21} \Delta x_2 \Delta x_1 + \omega_{12} \Delta x_1 \Delta x_2 = 0$$

Similarly, (23)

(31)

Thus: ω_{ij} does not affect length changes (rotation)

Comment: small $\frac{\partial u_i}{\partial x_j}$ means: both ϵ_{ij} and ω_{ij} small

This excludes: large rotations (at small strains)
(flexible structures)

Thus

$$\overline{\Delta S}^2 - \Delta S^2 = 2 \epsilon_{ij} \Delta x_i \Delta x_j$$

Transform this formula, to get relative elongation

$$\overline{\Delta S}^2 - \Delta S^2 = (\overline{\Delta S} - \Delta S) \underbrace{(\overline{\Delta S} + \Delta S)}_{\rightarrow 2\Delta S + (\overline{\Delta S} - \Delta S)}$$

Neglecting the term of second order in small difference $\overline{\Delta S} - \Delta S$:

$$\approx 2\Delta S (\overline{\Delta S} - \Delta S)$$

$$\Rightarrow \frac{\overline{\Delta S} - \Delta S}{\Delta S} = \frac{\epsilon_{ij} \Delta x_i \Delta x_j}{\Delta S^2}$$

Now, represent

$$\underline{\Delta x} = \Delta S \underline{n} \quad \leftarrow \text{orientation}$$

$$\Rightarrow \frac{\overline{\Delta S} - \Delta S}{\Delta S} = \epsilon_{ij} n_i n_j \quad \leftarrow \text{directional cosines}$$

Basic formula of strain analysis

Diagonal components of strain : for example ϵ_{11}

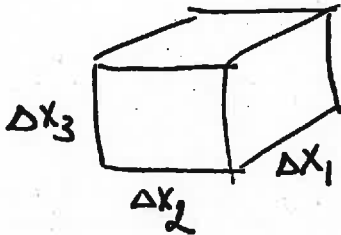
Choose mat'l element along X_1 axis : $n_1 = 1, n_2 = n_3 = 0$

Its relative elongation :

$$\frac{\bar{\Delta S} - \Delta S}{\Delta S} = \epsilon_{11}$$

$$\Rightarrow \text{its new length } \bar{\Delta S} = (1 + \epsilon_{11}) \Delta S$$

Sum of the diagonal elements ϵ_{ii} ($= \epsilon_{11} + \epsilon_{22} + \epsilon_{33}$)



$$\Delta V = \Delta x_1 \Delta x_2 \Delta x_3$$

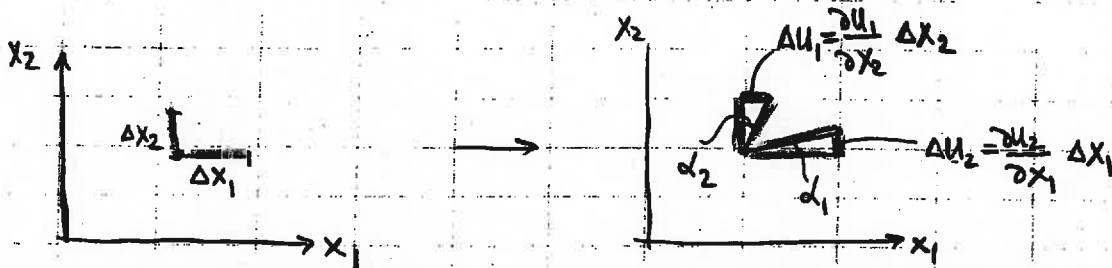
$$\bar{\Delta V} = \Delta x_1 (1 + \epsilon_{11}) \Delta x_2 (1 + \epsilon_{22}) \Delta x_3 (1 + \epsilon_{33})$$

$$= \Delta V (1 + \epsilon_{11} + \epsilon_{22} + \epsilon_{33} + \text{higher order terms})$$

$$\Rightarrow \epsilon_{ii} = \frac{\bar{\Delta V} - \Delta V}{\Delta V} \quad \text{-relative volume change}$$

(dilatation)

Off-Diagonal components of ϵ_{ij} . For example ϵ_{12}



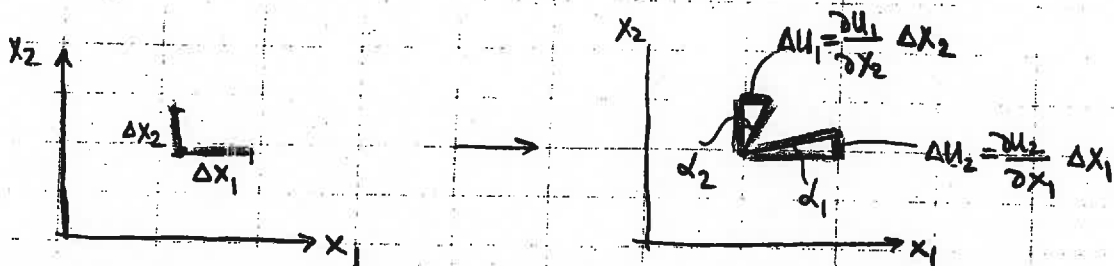
$$\tan \alpha_1 = \frac{\frac{\partial u_2}{\partial x_1} \Delta x_1}{\Delta x_1} = \frac{\partial u_2}{\partial x_1} \quad ; \quad \tan \alpha_2 = \frac{\partial u_1}{\partial x_2}$$

$$\therefore 2\epsilon_{12} = \frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} = \tan \alpha_1 + \tan \alpha_2 \approx \alpha_1 + \alpha_2 \quad \text{for small geom. changes}$$

$\therefore 2\epsilon_{12}$ = distortion of the (originally 90°) angle between material lines along x_1 & x_2 directions

Analogously, ϵ_{23} , ϵ_{31} .

Off-Diagonal components of ϵ_{ij} . For example ϵ_{12}



$$\tan \alpha_1 = \frac{\frac{\partial u_2}{\partial x_1} \Delta x_1}{\Delta x_1} = \frac{\partial u_2}{\partial x_1} ; \quad \tan \alpha_2 = \frac{\partial u_1}{\partial x_2}$$

$$\therefore 2\epsilon_{12} = \frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} = \tan \alpha_1 + \tan \alpha_2 \approx \alpha_1 + \alpha_2 \quad \text{for small geom changes}$$

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Analogously, ϵ_{23} , ϵ_{31} .

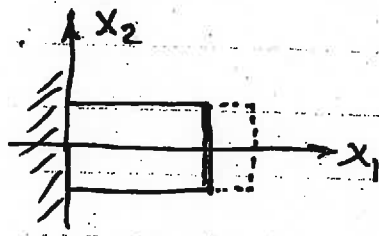
Principal Form of Strain

$$\epsilon_{ij} = \frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right) - \text{symmetric tensor} \quad \Rightarrow \quad 3 \text{ real roots in eigenv. problem}$$

$$\Rightarrow \underline{\epsilon} = \epsilon_I \underline{e}_I \underline{e}_I + \epsilon_{II} \underline{e}_{II} \underline{e}_{II} + \epsilon_{III} \underline{e}_{III} \underline{e}_{III}$$

Any deformation: equivalent to elongations & contractions in 3 principal directions; angles do not change

Example : uniaxial strain



↖ extent of straining

$$x_1 \rightarrow \bar{x}_1 = x_1 + kx_1$$

$$\bar{x}_2 = x_2$$

$$\bar{x}_3 = x_3$$

u_1

Displacements :

$$u_1 = kx_1$$

$$u_2 = u_3 = 0$$

Displac. gradient tensor:

$$D_{ij} = \frac{\partial u_i}{\partial x_j} = \begin{vmatrix} k & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{vmatrix}$$

- symmetric (no ω part)

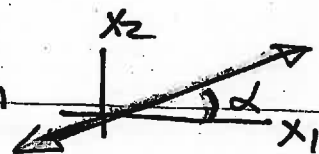
$$= \underline{\underline{\epsilon}}_{ij}$$

$$\epsilon_{11} = k, \text{ other } \epsilon_{ij} = 0$$

In dyadic form:

$$\underline{\underline{\epsilon}} = k \underline{e}_1 \underline{e}_1$$

Now, find relative elongation in direction



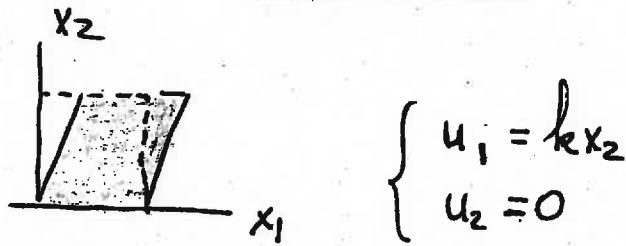
$$\text{Unit vector : } \underline{n} = \underbrace{\cos \alpha}_{n_1} \underline{e}_1 + \underbrace{\sin \alpha}_{n_2} \underline{e}_2$$

$$\frac{\Delta \bar{S} - \Delta S}{\Delta S} = \epsilon_{ij} n_i n_j = \epsilon_{11} n_1^2 = k \cos^2 \alpha$$

easily done using tensors

Note: independent of $\underline{n} \rightarrow -\underline{n}$

Example : Shear strain



$$D_{ij} = \frac{\partial u_i}{\partial x_j} = \begin{vmatrix} 0 & k \\ 0 & 0 \end{vmatrix}$$

$$\underline{\underline{\epsilon}}_{ij} = \begin{vmatrix} 0 & k/2 \\ k/2 & 0 \end{vmatrix}$$

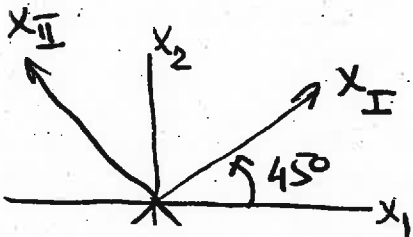
$$\underline{\underline{\epsilon}} = \frac{k}{2} (\underline{e}_1 \underline{e}_2 + \underline{e}_2 \underline{e}_1)$$

No volume change!

Eigenvalue problem :

$$\det \begin{vmatrix} -\lambda & k/2 \\ k/2 & -\lambda \end{vmatrix} = 0 \Rightarrow \lambda_{1,2} = \pm k/2$$

$$\text{Eigenvectors: } \lambda_1 = \frac{k}{2} \quad \begin{cases} -\frac{k}{2}x_1 + \frac{k}{2}x_2 = 0 \\ \frac{k}{2}x_1 - \frac{k}{2}x_2 = 0 \end{cases}$$



$$\Rightarrow x_1 = x_2$$

$$\lambda_2 = -k/2 \Rightarrow x_1 = -x_2$$

Principal axes representation $\underline{\underline{\epsilon}} = \frac{k}{2} (\underline{e}_I \underline{e}_I - \underline{e}_{II} \underline{e}_{II})$ - shear strain is equiv. to stretch & contract at 45° direction

relative elongation in direction

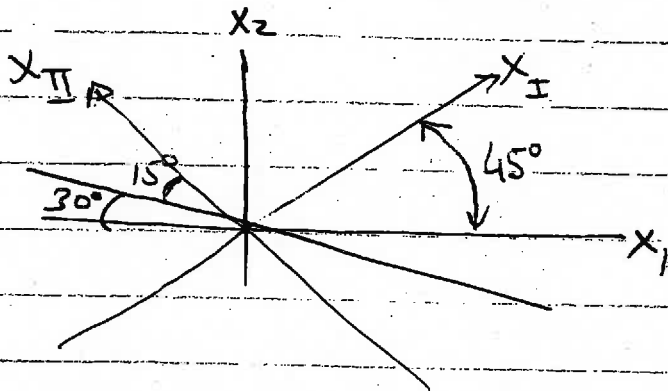


$$\text{Unit vector: } \underbrace{-\frac{\sqrt{3}}{2}}_{n_1} \underline{e}_1 + \underbrace{\frac{1}{2}}_{n_2} \underline{e}_2 = \underline{n}$$

$$\frac{\Delta \bar{S} - \Delta S}{\Delta S} = \underline{\underline{\epsilon}}_{ij} n_i n_j = \frac{k}{2} (n_1 n_2 + n_2 n_1) = -\frac{\sqrt{3}}{4} k$$

↙ contraction

the same calculation in the principal axes



$$\epsilon_I = \frac{k}{2} \quad \epsilon_{II} = -\frac{k}{2}$$

$$\underline{\epsilon} = \frac{k}{2} (\underline{e}_I \underline{e}_I - \underline{e}_{II} \underline{e}_{II})$$

Unit vector of the same direction in the princ. coord. system:

$$\underline{n} = \underbrace{-\sin 15^\circ}_{n_I} \underline{e}_I + \underbrace{\cos 15^\circ}_{n_{II}} \underline{e}_{II}$$

$$\frac{\overline{\Delta S} - \Delta S}{\Delta S} = \underbrace{\epsilon_I n_I n_I}_{\epsilon_I n_I^2} + \epsilon_{II} n_{II}^2 =$$

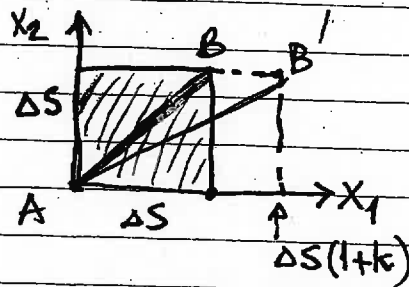
$$= \frac{k}{2} (\underbrace{\sin^2 15^\circ - \cos^2 15^\circ}_{-\cos 30^\circ}) = -\frac{\sqrt{3}}{4} k - s_0$$

Note on Smallness of Strains & Rotations

We assume all $|\frac{\partial u_i}{\partial x_j}| \ll 1$ (not only ϵ_{ij} , but ω_{ij} as well)

Only then $(\epsilon_{ij}, \omega_{ij})$ have the meanings we identified

Example uniaxial elongation



$$u_1 = kx_1 \quad u_2 = u_3 = 0 \quad \Rightarrow \quad \epsilon_{11} = k$$

Elongation of diagonal (Pythagorean theorem):

$$AB' - AB = [\sqrt{(1+k)^2 + 1} - \sqrt{2}] \Delta S$$

Relative elong:

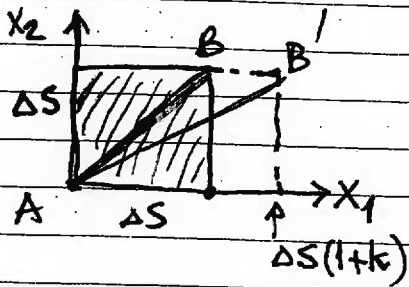
$$\frac{[\sqrt{(1+k)^2 + 1} - \sqrt{2}] \Delta S}{(\Delta S \sqrt{2})}$$

Note on Smallness of Strains & Rotations

We assume all $|\frac{\partial u_i}{\partial x_j}| \ll 1$ (not only ϵ_{ij} , but ω_{ij} as well)

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$$AB' - AB = [\sqrt{(1+k)^2 + 1} - \sqrt{2}] \Delta s$$

Relative elong:
$$\frac{[\sqrt{(1+k)^2 + 1} - \sqrt{2}] \Delta s}{(\Delta s \sqrt{2})}$$

From strains:

$$\epsilon_{11} = k, \text{ other } \epsilon_{ij} = 0$$

relative elong:
$$\epsilon_{ij} n_i n_j = \epsilon_{11} \left(\frac{1}{\sqrt{2}}\right)^2 = \frac{1}{2} k \quad \text{- differs}$$

But, if k is small - the same

Indeed:
$$\sqrt{(1+k)^2 + 1} \approx \sqrt{2 + 2k} = \sqrt{2} \sqrt{1+k}$$
 small k

recall:
$$\sqrt{1+k} \approx 1 + \frac{1}{2} k \quad \text{(Taylor series)}$$

\Rightarrow Pythagorean theorem yields $\frac{1}{2} k$

What if ϵ_{ij} and ω_{ij} are not small?
then $\frac{\partial u_i}{\partial x_j}$ quantities do not mean anything

Processing of field data : finding strains
from observed data

Observed data geometry changes for several orientations of material lines

- elongations
- angle changes

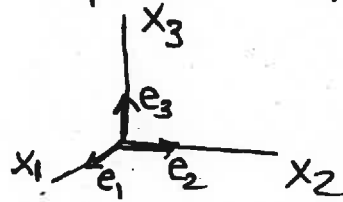
Want : strain tensor

Motivation: analyze material lines
of any orientation

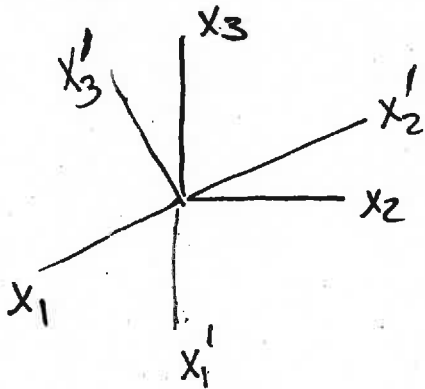
Tool needed : given strain components in one coord. system
find them in another (rotated) system

Strain tensor $\underline{\underline{\epsilon}} = \epsilon_{ij} \underline{e}_i \underline{e}_j$

components in system x_1, x_2, x_3



Components of $\underline{\underline{\epsilon}}$ in x'_1, x'_2, x'_3 ?



For vectors :

$$\underline{a} = a_i \underline{e}_i$$

express in terms of $\underline{e}'_1, \underline{e}'_2, \underline{e}'_3$

$$\underline{e}_i = \alpha_{ij} \underline{e}'_j$$

direct. cosines

$$\Rightarrow \underline{a} = a_i \alpha_{ij} \underline{e}'_j$$

j -th comp. in the (new) system

$$\dots + \underbrace{(a_i \alpha_{i3})}_{a'_3} \underline{e}'_3 + \dots$$

$$\alpha_{23} = \cos(\hat{e}_2 \hat{e}'_3)$$

Tensors: similarly

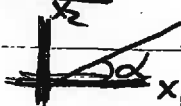
$$\underline{\underline{\epsilon}} = \epsilon_{ij} \underline{e}_i \underline{e}_j$$

express in terms of $\underline{e}'_1, \underline{e}'_2, \underline{e}'_3$

$$= \underbrace{\epsilon_{ij} \alpha_{ik} \alpha_{jl}}_{\epsilon'_{kl}} \underline{e}'_k \underline{e}'_l$$

Now we can transfer info. collected from various orientations to a chosen fixed coord system X_1, X_2

A. Transfer of elongation info.

Let us say relative elongation has been measured = δ in some direction α 

$$\Rightarrow \delta = \epsilon_{11} \cos^2 \alpha + \epsilon_{22} \sin^2 \alpha + 2 \sin \alpha \cos \alpha \epsilon_{12} = \epsilon'_{11}$$

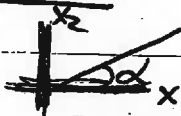
one eq-n for 3 unknowns ϵ_{ij}

If δ 's are measured in 3 directions - can find all ϵ_{ij} .

Now we can transfer info. collected from various orientations to a chosen fixed coord system x_1, x_2

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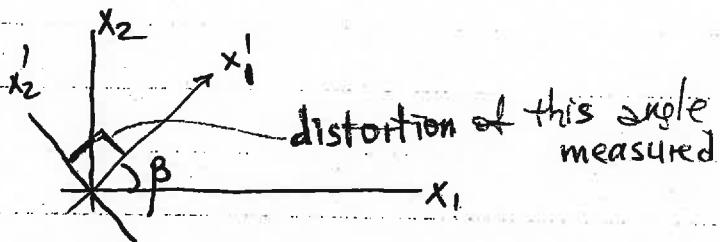


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one eq-n for 3 unknowns ϵ_{ij}

If δ 's are measured in 3 directions - can find all ϵ_{ij} .

B. Transfer of angle distortion info



$$\epsilon'_{12} = \left(\frac{1}{2} \text{angle change} \right) = (\epsilon_{22} - \epsilon_{11}) \sin \beta \cos \beta + \epsilon_{12} (\cos^2 \beta - \sin^2 \beta)$$

↑ only the difference enters!

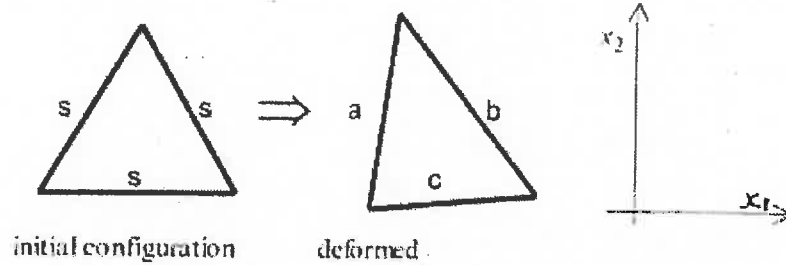
⇒ reconstruction of ϵ_{ij} is incomplete
(in contrast with info. on elongations)

[Example: $\underline{\epsilon} = \delta \underline{I}$ → no angular distortions → cannot find δ]

(B)

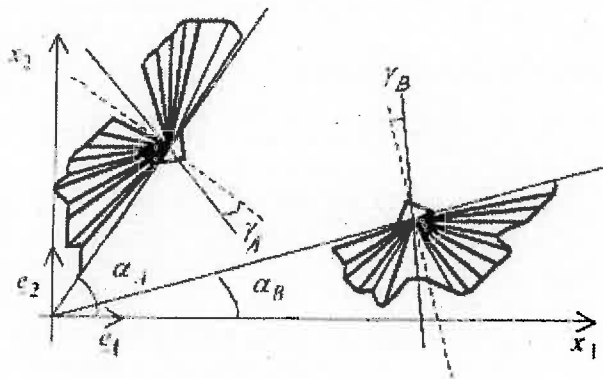
Construction of strains from measured data (2-D problems)

1. Three stakes are set in the surface of a glacier to form an equilateral triangle. The glacier "creeps" slowly, and the positions of the stakes are monitored, to estimate the (accumulated) strain. After some time, the triangle is re-surveyed and found to be as follows:



Find the strains accumulated by ice, i.e. find all strain components in x_1, x_2 coordinate system in terms of s, a, b, c and then specify them for the following data: $s = 10\text{m}$, $a = 10.1\text{m}$, $b = 10.4\text{m}$, $c = 9.4\text{m}$.

2. Two fossils of different orientations, α_A, α_B with respect to some given fixed coordinate system x_1, x_2 were found in geological strata. The deformed state is characterized by γ_A and γ_B (the changes of 90° angles between fossil "axes")



Try to determine strains ϵ_{ij} in the strata. You may not be able to fully determine ϵ_{ij} ; find, to what extent ϵ_{ij} can be determined.

- Will this uncertainty be reduced if similar information on one more (third) fossil becomes available?
- Discuss the difference with problem #3 – why ϵ_{ij} can be fully determined there, and only partially here?

Stresses

Body forces and surface forces (tractions)

Body force : applied to elements of mass (gravity)

$$\underline{\underline{b}} \Delta m \text{ - force on } \Delta m$$

↑ force/unit mass (= g with direction)

$$= \underline{\underline{b}} \rho \Delta V$$

force/unit volume

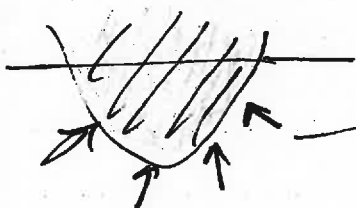
Total force on volume V : $\int_V \underline{\underline{b}} \rho dV$

Surface force : applied to elements of surface

$$\underline{\underline{t}} \Delta S$$

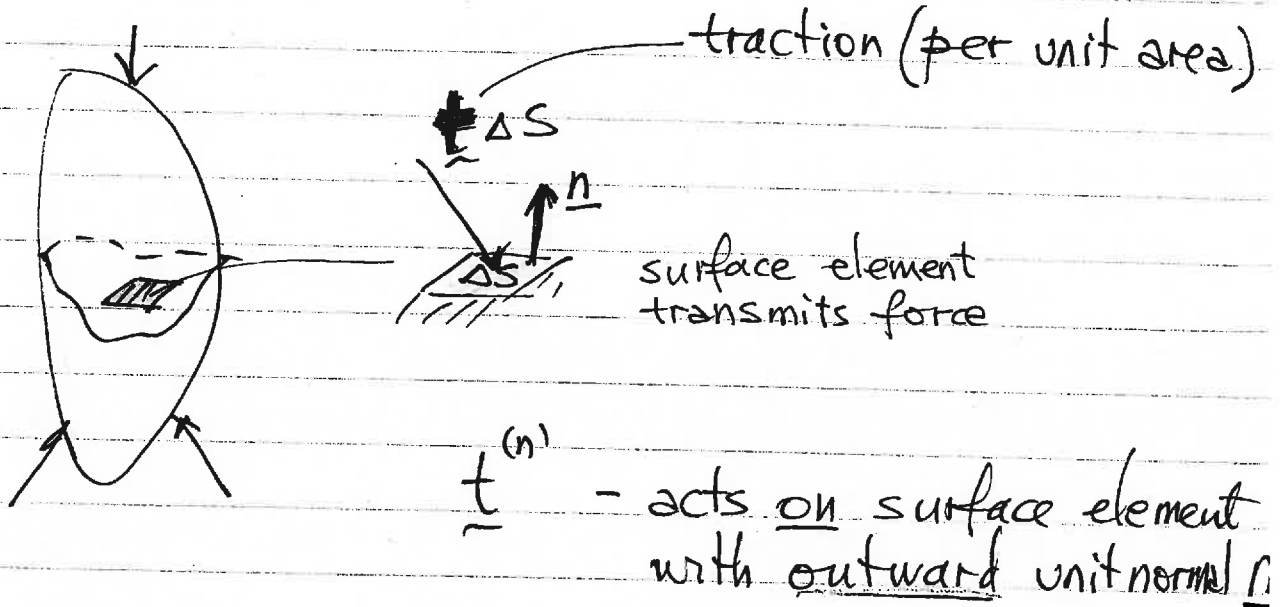
↑ traction (force per unit area)

Example : bouyancy force



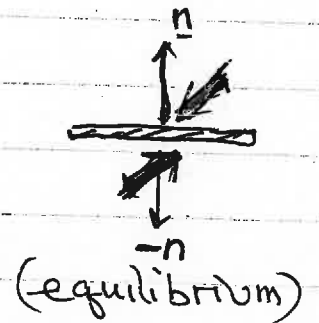
pressure from fluid
(normal to surface)

stressed solid



Obviously,

$$\underline{t}^{(-n)} = -\underline{t}^{(n)}$$

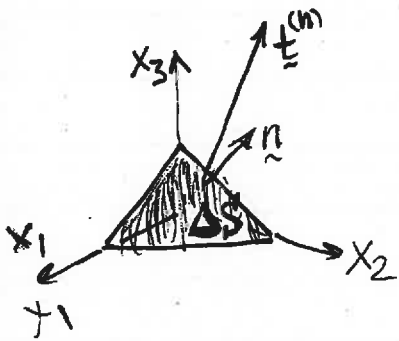


traction $\underline{t}^{(n)}$ depends on orientation \underline{n} .

examine this dependence, $\underline{t}^{(n)} = \underline{t}^{(n)}(\underline{n})$

Equilibrium of volume element

To examine orient. dependence of traction $\underline{t}^{(n)}$: consider small tetrahedron



bounded by: - coordinate planes:

$$\Delta S_i = \Delta S \underline{n} \cdot \underline{e}_i$$

- inclined plane ΔS

Principal vector of all forces appl. to tetrahedron:

$$\underline{t}^{(n)} \Delta S + \underline{t}^{(-i)} \Delta S (\underline{e}_i \cdot \underline{n}) + b \rho \Delta V = 0$$

Divide over ΔS and use $\underline{t}^{(-i)} = -\underline{t}^{(i)}$

$$\underline{t}^{(n)} = \underline{t}^{(i)} (\underline{e}_i \cdot \underline{n}) - b \rho \frac{\Delta V}{\Delta S} \rightarrow 0 \text{ for small element}$$

$$\underline{t}^{(n)} = \underbrace{(\underline{t}^{(1)} \underline{e}_1 + \dots)}_{\underline{\sigma}} \cdot \underline{n}$$

$\underline{\sigma}$ (as a sum of three dyads)

$$\therefore \underline{t}^{(n)} = \underline{\sigma} \cdot \underline{n} \quad \text{- linear vector f-n of vector arg}$$

\nearrow traction vector \nearrow stress tensor

Meaning of stress components

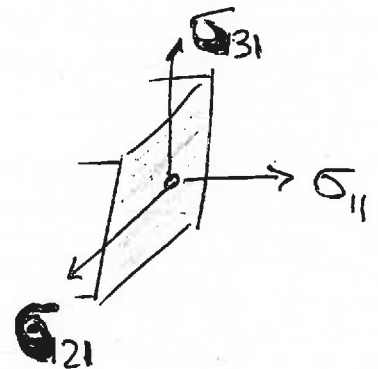
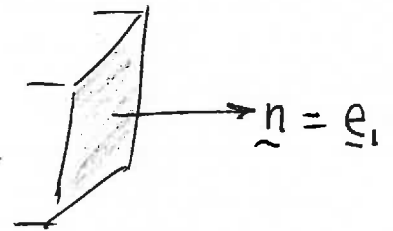
$$\vec{\sigma} \cdot \vec{n} = t^{(n)}$$

$\sigma_{ij} \underline{e}_i \underline{e}_j$

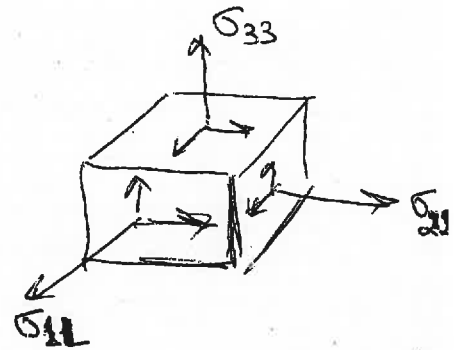
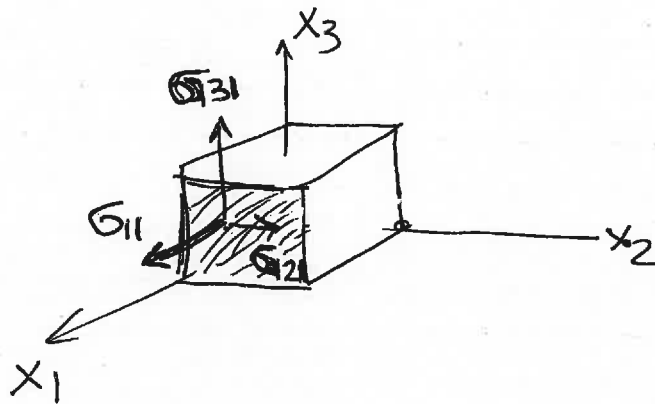
Choose $\vec{n} = \underline{e}_1$

$$t^{(n)} = \sigma_{ij} \underline{e}_i \underline{e}_j \cdot \underline{e}_1$$

$$= \sigma_{i1} \underline{e}_i$$



\therefore Stress components, in a given coord. system:



Diagonal: $\sigma_{11}, \sigma_{22}, \sigma_{33} > 0$ tensile
 < 0 compressive

Off-diagonal: $\sigma_{12}, \sigma_{23}, \sigma_{31}$ - shear stresses

$$p = \frac{1}{3} (\sigma_{11} + \sigma_{22} + \sigma_{33}) \quad \text{— ave. hydrostatic stress}$$

In fluids (ideal fluid (no viscosity) or viscous fluid in statics)

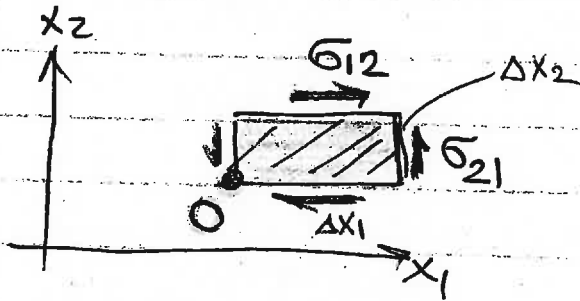
$$\underline{\underline{\sigma}} = -p \underline{\underline{I}}$$

implies: $\underline{\underline{\sigma}} \cdot \underline{\underline{n}} = -p \underline{\underline{n}}$

(Pascal's law)

same normal pressure in
all directions

Second equilibrium eq: total angular mom = 0



Taking Mom_0 :

$$\sigma_{12} \Delta x_1 \Delta x_2 = \sigma_{21} \Delta x_1 \Delta x_2$$

$$\Rightarrow \sigma_{12} = \sigma_{21}$$

Generally:

$$\sigma_{ij} = \sigma_{ji}$$

- stress tensor is symm

→ 3 real eigenvalues

$$\Rightarrow \sigma = \sigma_I \underline{e}_I \underline{e}_I + \dots$$



normal only

equilibrium of small element:

princ. vector = 0



stress tensor introduced

$\underline{t}^{(n)} = \underline{\sigma} \cdot \underline{n}$

orient. dependence of tractions

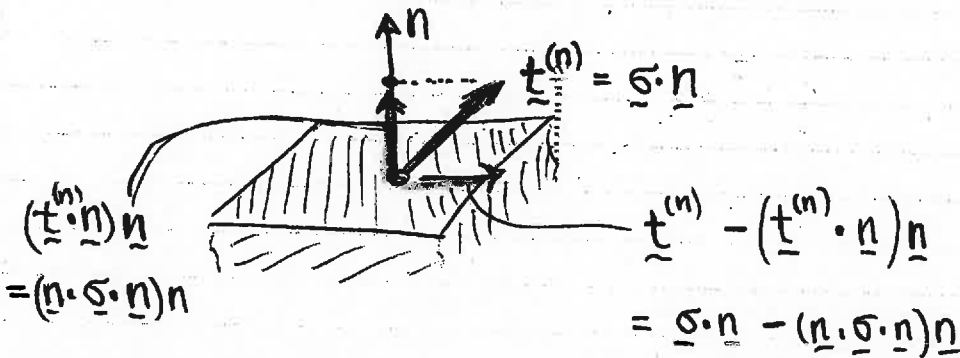
ang. mom. = 0



$\sigma_{ij} = \sigma_{ji}$

stress tensor is symm; three real eigenvalues

Normal and shear components of $\underline{t}^{(n)}$ (traction vector)

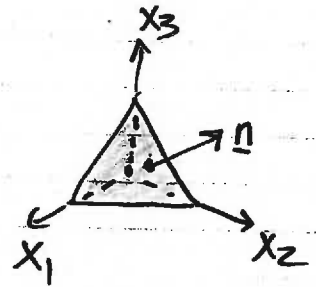


In components :

$$\left\{ \begin{array}{l} \underline{n} \cdot \underline{\sigma} \cdot \underline{n} = \sigma_{ij} n_i n_j \\ \underline{t}^{(n)} = \sigma_{ij} n_j \underline{e}_i \\ \underline{t}^{(n)} \cdot \underline{t}^{(n)} = \sigma_{ij} \sigma_{jk} n_i n_k = \underline{n} \cdot \underline{\sigma} \cdot \underline{\sigma} \cdot \underline{n} \end{array} \right.$$

Numerical example on normal & shear tractions

Shear stress: $\underline{\sigma} = \tau (\underline{e}_1 \underline{e}_2 + \underline{e}_2 \underline{e}_1)$



Tractions induced on the plane

forming equal angles with x_1, x_2, x_3

$$\underline{n} = \frac{1}{\sqrt{3}} (\underline{e}_1 + \underline{e}_2 + \underline{e}_3)$$

not 45° !
 $\arccos 1/\sqrt{3} \approx 55^\circ$

$$\underline{t}^{(n)} = \underline{\sigma} \cdot \underline{n} = \frac{\tau}{\sqrt{3}} (\underline{e}_1 + \underline{e}_2) = \underbrace{\tau \sqrt{\frac{2}{3}}}_{\text{magnitude}} \underbrace{\left(\frac{\underline{e}_1}{\sqrt{2}} + \frac{\underline{e}_2}{\sqrt{2}} \right)}_{\text{unit vector}}$$

Normal comp:

$$\underline{t}^{(n)} \cdot \underline{n} = \frac{\tau}{\sqrt{3}} (\underline{e}_1 + \underline{e}_2) \cdot \frac{1}{\sqrt{3}} (\underline{e}_1 + \underline{e}_2 + \underline{e}_3) = \frac{2}{3} \tau$$

Shear

$$\begin{aligned} \underline{t}^{(n)} - (\underline{t}^{(n)} \cdot \underline{n}) \underline{n} &= \frac{\tau}{\sqrt{3}} (\underline{e}_1 + \underline{e}_2) - \frac{2}{3} \tau \cdot \frac{1}{\sqrt{3}} (\underline{e}_1 + \underline{e}_2 + \underline{e}_3) \\ &= \tau \underbrace{\frac{\sqrt{2}}{3}}_{\text{magn.}} \underbrace{\left(\frac{1}{\sqrt{6}} \underline{e}_1 + \frac{1}{\sqrt{6}} \underline{e}_2 - \frac{2}{\sqrt{6}} \underline{e}_3 \right)}_{\text{unit vector}} \end{aligned}$$

Check

$$\underline{t}^{(n)} \cdot \underline{t}^{(n)} = (\text{must be}) = \left(\frac{2}{3} \tau \right)^2 + \left(\tau \cdot \frac{\sqrt{2}}{3} \right)^2 \quad (\text{Pythagorean th})$$

• shear traction must be $\perp \underline{n}$



$$\left(\frac{1}{\sqrt{6}} \underline{e}_1 + \frac{1}{\sqrt{6}} \underline{e}_2 - \frac{2}{\sqrt{6}} \underline{e}_3 \right) \cdot \underline{n} = 0$$

Elastic Anisotropy

(elastic properties are different in different directions)

Most materials are anisotropic!

- Crystals
- rocks
- composites
- wood
-

Hooke's law: linear relations between ϵ_{ij} & σ_{ij}

Isotropic material (same properties in all directions):

$$\left\{ \begin{array}{l} \epsilon_{11} = \frac{1}{E} \sigma_{11} - \frac{\nu}{E} (\sigma_{22} + \sigma_{33}) \\ \epsilon_{22} = \frac{1}{E} \sigma_{22} - \frac{\nu}{E} (\sigma_{11} + \sigma_{33}) \\ \epsilon_{33} = \frac{1}{E} \sigma_{33} - \frac{\nu}{E} (\sigma_{11} + \sigma_{22}) \end{array} \right. \quad \left\{ \begin{array}{l} \epsilon_{12} = \frac{1}{2G} \sigma_{12} \\ \epsilon_{23} = \frac{1}{2G} \sigma_{23} \\ \epsilon_{31} = \frac{1}{2G} \sigma_{31} \end{array} \right.$$

Of 3 elastic constants E , ν , G only 2 are independent

$$G = \frac{E}{2(1+\nu)}$$

Anisotropic Materials : Hooke's law

Each ϵ_{ij} is a linear f-n of all σ_{ij} :

$$\epsilon_{ij} = S_{ijkl} \sigma_{kl}$$

compliance tensor, 4th rank
same summation convention

Isotropic

Case: $S_{1111} = \frac{1}{E}$, $S_{1122} = -\frac{\nu}{E}$, $S_{1212} = \frac{1}{4G}$

(etc.)

[note: $\epsilon_{12} = S_{1212} \sigma_{12} + S_{1221} \sigma_{21}$]

Anisotropic Materials : Hooke's law

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(etc.)

[note: $\epsilon_{12} = S_{1212} \sigma_{12} + S_{1221} \sigma_{21}$]

Number of independent compliances:

$$9^2 = 81$$

However: \rightarrow Since $\epsilon_{ij} = \epsilon_{ji}$, $\sigma_{kl} = \sigma_{lk}$

$$6^2 = 36$$

\rightarrow Since $S_{ijkl} = S_{klij} \Rightarrow$ 21

number of elastic constants

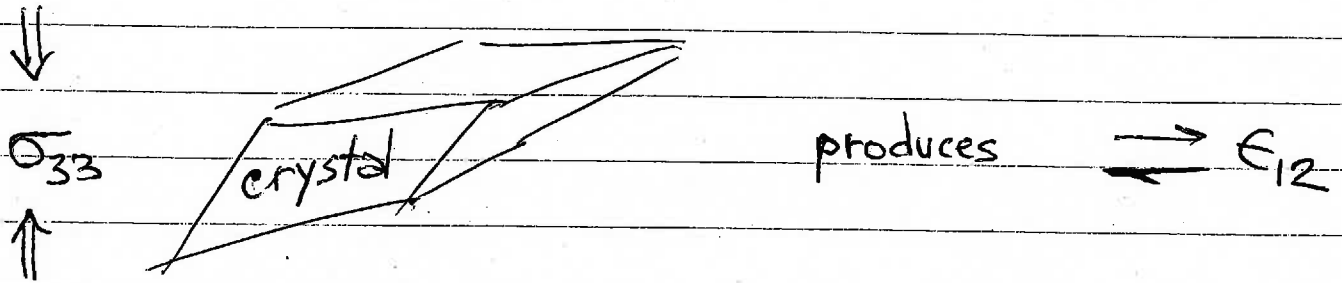
Alternatively:

$$\sigma_{ij} = C_{ijkl} \epsilon_{kl}$$

stiffnesses.

In anisotropic materials, normal & shear modes may be coupled:

If $S_{1233} \neq 0 \Rightarrow \epsilon_{12}$ depends on σ_{33}



Thus,

$$\epsilon_{ij} = S_{ijkl} \sigma_{kl}$$

↑ 21 constants (compliances)

Material symmetries greatly reduce the number of constants

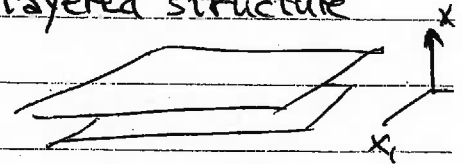
important cases:

- Orthotropy
(rectangular symm.)

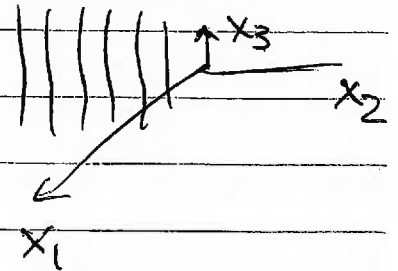


- Transverse isotropy
(isotropy within x_1, x_2 plane)

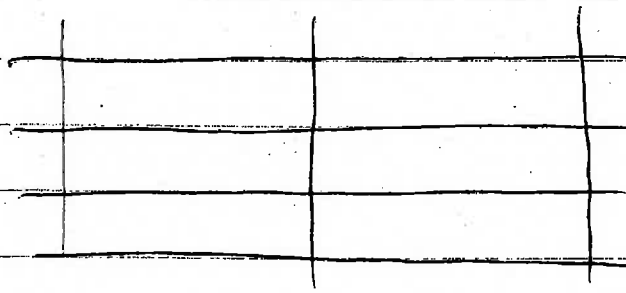
- layered structure



- parallel fibers



Orthotropy in 2-D



↑
↔ principal axes of orthotropy

$$\epsilon_{11} = S_{1111} \sigma_{11} + S_{1122} \sigma_{22}$$

$$\epsilon_{22} = S_{2211} \sigma_{11} + S_{2222} \sigma_{22}$$

$$\epsilon_{12} = S_{1212} \sigma_{12} + S_{1221} \sigma_{21}$$

$$= 2 S_{1212} \sigma_{12}$$

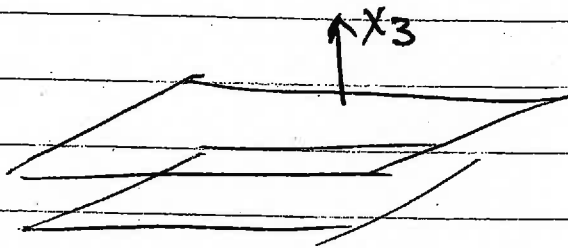
3-D orthotropy :

$$\left\{ \begin{aligned} \epsilon_{11} &= \frac{1}{E_1} \sigma_{11} - \frac{\nu_{21}}{E_2} \sigma_{22} - \frac{\nu_{31}}{E_3} \sigma_{33} \\ \epsilon_{22} &= -\frac{\nu_{12}}{E_1} \sigma_{11} + \frac{1}{E_2} \sigma_{22} - \frac{\nu_{32}}{E_3} \sigma_{33} \\ \epsilon_{33} &= -\frac{\nu_{13}}{E_1} \sigma_{11} - \frac{\nu_{23}}{E_2} \sigma_{22} + \frac{1}{E_3} \sigma_{33} \end{aligned} \right.$$

$$\left\{ \begin{aligned} \epsilon_{12} &= \frac{1}{2G_{12}} \sigma_{12} \\ \epsilon_{23} &= \frac{1}{2G_{23}} \sigma_{23} \\ \epsilon_{31} &= \frac{1}{2G_{31}} \sigma_{31} \end{aligned} \right.$$

9 constants
(independent)

Transverse Isotropy



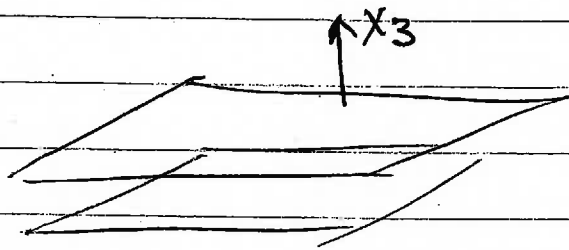
$x_1 x_2$ - plane of isotropy

Hooke's law $\epsilon_{ij} = S_{ijkl} \sigma_{kl}$ takes form

plane of isotropy

$$\left. \begin{aligned} \epsilon_{11} &= \frac{1}{E_0} \sigma_{11} - \frac{\nu_0}{E_0} \sigma_{22} - \left(\frac{\nu_{31}}{E_3} \right) \sigma_{33} \\ \epsilon_{22} &= -\frac{\nu_0}{E_0} \sigma_{11} + \frac{1}{E_0} \sigma_{22} - \left(\frac{\nu_{32}}{E_3} \right) \sigma_{33} \\ \epsilon_{33} &= -\left(\frac{\nu_{13}}{E_0} \right) \sigma_{11} - \left(\frac{\nu_{23}}{E_0} \right) \sigma_{22} + \frac{1}{E_3} \sigma_{33} \end{aligned} \right\}$$

Transverse Isotropy



plane x_1x_2 - plane of isotropy

Hooke's law $\epsilon_{ij} = S_{ijkl} \sigma_{kl}$ takes form:

of plane of isotropy

$$\left. \begin{aligned} \epsilon_{11} &= \frac{1}{E_0} \sigma_{11} - \frac{\nu_0}{E_0} \sigma_{22} - \left(\frac{\nu_{31}}{E_3}\right) \sigma_{33} \\ \epsilon_{22} &= -\frac{\nu_0}{E_0} \sigma_{11} + \frac{1}{E_0} \sigma_{22} - \left(\frac{\nu_{32}}{E_3}\right) \sigma_{33} \\ \epsilon_{33} &= -\left(\frac{\nu_{13}}{E_0}\right) \sigma_{11} - \left(\frac{\nu_{23}}{E_0}\right) \sigma_{22} + \frac{1}{E_3} \sigma_{33} \end{aligned} \right\}$$

$$\left. \begin{aligned} \epsilon_{12} &= \frac{1}{2G_{10}} \sigma_{12} \\ \epsilon_{23} &= \frac{1}{2G_{23}} \sigma_{23} \\ \epsilon_{31} &= \frac{1}{2G_{31}} \sigma_{31} \end{aligned} \right\}$$

5 elastic (independent) constants

$$E_0, \nu_0 \quad (G_0 = E_0 / 2(1 + \nu_0))$$

$$E_3, \nu_{13}, G_{23}$$

Important consequence of anisotropy:

Hydrostatic loading produces shear strains

Example: 2-D orthotropy

$$\left\{ \begin{array}{l} \epsilon_{11} = \frac{1}{E_1} \sigma_{11} - \left(\frac{\nu_{21}}{E_2} \right) \sigma_{22} \\ \epsilon_{22} = - \left(\frac{\nu_{12}}{E_1} \right) \sigma_{11} + \frac{1}{E_2} \sigma_{22} \\ \epsilon_{12} = \frac{1}{2G_{12}} \sigma_{12} \end{array} \right.$$

Apply: $\sigma_{11} = \sigma_{22} = p$; $\sigma_{12} = 0$ (hydro. loading)

$$\epsilon_{11} = \left(\frac{1}{E_1} - \frac{\nu_{21}}{E_2} \right) p$$

$$\epsilon_{22} = \left(\frac{1}{E_2} - \frac{\nu_{12}}{E_1} \right) p \neq \epsilon_{11} !$$

Since $\epsilon_{11} \neq \epsilon_{22}$, shear strains induced on some orient's

$$\epsilon'_{12} = (\epsilon_{22} - \epsilon_{11}) \sin\beta \cos\beta + \underbrace{\epsilon_{12}}_0 (\cos^2\beta - \sin^2\beta)$$

Used for: restructuring of crystals (graphite \rightarrow diamond)
by high pressures

Handling of Anisotropy

requires

Tensors of 4th rank

Strains ϵ_{ij} } - tensors of second rank
 Stresses σ_{ij} }

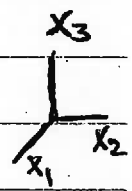
Compliances S_{ijkl} } - tensors of
 Stiffnesses C_{ijkl} } fourth rank

Working with them?

Key point: represent them in dyadic form

$$\underline{\underline{S}} = S_{ijkl} \underline{e}_i \underline{e}_j \underline{e}_k \underline{e}_l$$

$$\underline{\underline{C}} = C_{ijkl} \underline{e}_i \underline{e}_j \underline{e}_k \underline{e}_l$$



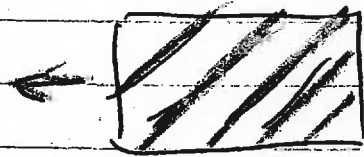
↑ objective quantities ↑ components is a chosen coord system

Compare with:

$$\underline{\underline{\epsilon}} = \epsilon_{ij} \underline{e}_i \underline{e}_j$$

$$\underline{\underline{\sigma}} = \sigma_{ij} \underline{e}_i \underline{e}_j$$

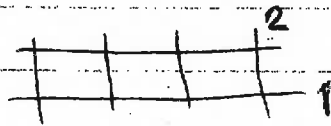
Elastic constants in arbitrary directions



stiffness in this direction?

In the principal axes of symmetry:

2D orthotropic:



$$\left\{ \begin{aligned} \epsilon_{11} &= \frac{1}{E_1} \sigma_{11} - \frac{\nu_{21}}{E_2} \sigma_{22} \\ \epsilon_{22} &= -\frac{\nu_{12}}{E_1} \sigma_{11} + \frac{1}{E_2} \sigma_{22} \\ \epsilon_{12} &= \frac{1}{2G_{12}} \sigma_{12} \end{aligned} \right.$$

Approach:

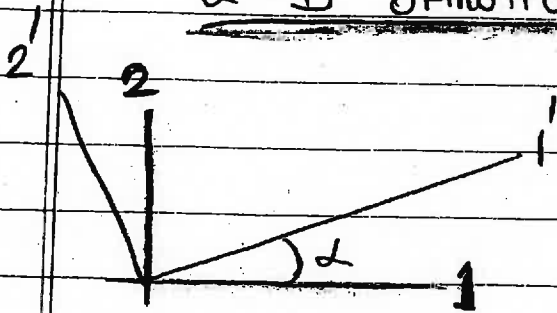
$$\underline{\underline{S}} = S_{ijkl} e_i e_j e_k e_l$$

change ($e_i \rightarrow e'_i$)

Similar to strains:

$$\begin{aligned} \epsilon &= \epsilon_{ij} e_i e_j \\ &= \epsilon'_{ij} e'_i e'_j \end{aligned}$$

2-D orthotropic material



x_1, x_2 - principal axes of orthotropy

Compliance tensor in the principal axes

$$\underline{\underline{S}} = \underbrace{\frac{1}{E_1}}_{S_{1111}} \underline{e}_1 \underline{e}_1 \underline{e}_1 \underline{e}_1 + \underbrace{\frac{1}{E_2}}_{S_{2222}} \underline{e}_2 \underline{e}_2 \underline{e}_2 \underline{e}_2 +$$

$$+ \underbrace{\frac{1}{4G_{12}}}_{S_{1212} = S_{1221} = S_{2112} = S_{2121}} (\underline{e}_1 \underline{e}_2 \underline{e}_1 \underline{e}_2 + \underline{e}_1 \underline{e}_2 \underline{e}_2 \underline{e}_1 + \underline{e}_2 \underline{e}_1 \underline{e}_1 \underline{e}_2 + \underline{e}_2 \underline{e}_1 \underline{e}_2 \underline{e}_1)$$

[note: $\epsilon_{12} = \frac{1}{4G_{12}} (\sigma_{12} + \sigma_{21}) = \frac{1}{2G_{12}} \sigma_{12}$]

$$S_{1212} = S_{1221} \\ = S_{2112} = S_{2121}$$

$$= -\frac{\sqrt{21}}{E_2}$$

$$- \frac{\sqrt{12}}{E_1} (\underline{e}_1 \underline{e}_1 \underline{e}_2 \underline{e}_2 + \underline{e}_2 \underline{e}_2 \underline{e}_1 \underline{e}_1)$$

$$S_{1122} = S_{2211}$$

Rotate axes

$$\begin{cases} e_1 = \cos \alpha e'_1 - \sin \alpha e'_2 \\ e_2 = \sin \alpha e'_1 + \cos \alpha e'_2 \end{cases}$$

substitute
into $S_{ijkl} e_i e_j e_k e_l$

$$\Rightarrow S'_{ijke} e'_i e'_j e'_k e'_e$$

Find S'_{1111}
(coeff. at $e'_1 e'_1 e'_1 e'_1$)

contribution from S_{1111} - term:

$$S_{1111} e_1 e_1 e_1 e_1$$

$$e_1 = \cos \alpha e'_1 - \sin \alpha e'_2$$

$$\Rightarrow S_{1111} \cos^4 \alpha$$

contribution from S_{2222} term:

$$S_{2222} e_2 e_2 e_2 e_2$$

$$e_2 = \sin \alpha e'_1 + \cos \alpha e'_2$$

$$\Rightarrow S_{2222} \sin^4 \alpha$$

Similarly:

$$\underbrace{S_{1122}}_{\leftarrow} \cos^2 \alpha \sin^2 \alpha + \underbrace{S_{2211}}_{\rightarrow} \sin^2 \alpha \cos^2 \alpha$$

← equal →

$$+ S_{1212} \sin^2 \alpha \cos^2 \alpha \cdot 4$$

from: 1212, 1221, 2112, 2121

Now, identify

$$S_{1111} = \frac{1}{E_1}$$

$$S'_{1111} = \frac{1}{E'_1}$$

$$S_{1122} = S_{2211} = -\frac{\nu_{12}}{E_1} = -\frac{\nu_{21}}{E_2}$$

$$S_{1212} = \frac{1}{4G_{12}}$$

comes from:

$$\begin{aligned} \epsilon_{12} &= S_{1212} \sigma_{12} + S_{1221} \sigma_{21} = \\ &= 2S_{1212} \sigma_{12} \\ &= \frac{1}{2G_{12}} \sigma_{12} \end{aligned}$$

($\epsilon_{12kl} \sigma_{kl}$) ←

⇒

$$\frac{1}{E'_1} = \frac{1}{E_1} \cos^4 \alpha + \frac{1}{E_2} \sin^4 \alpha + \frac{1}{4G_{12}} \cdot \sin^2 \alpha \cos^2 \alpha \cdot 4$$

$$- \frac{\nu_{12}}{E_1} \cdot \sin^2 \alpha \cos^2 \alpha \cdot 2$$

(axial stiffness (Young's modulus)
as function of direction